



**THE EFFECT OF U-TURN ON TRAFFIC FLOW CHARACTERISTICS (A CASE STUDY: JENDRAL BASUKI RACHMAT ROAD, EAST JAKARTA)**

Basit Al Hanif, Noviandi Putra Darsono

<sup>1</sup>Civil Engineering Study Program, University of Muhammadiyah Jakarta, Jl. Cempaka Putih Tengah 27, Indonesia

Correspondence email: basit.alhanif@umj.ac.id

<sup>2</sup>Civil Engineering Study Program, University of Muhammadiyah Jakarta, Jl. Cempaka Putih Tengah 27, Indonesia

Email: noviandi.putra@staff.umj.ac.id

Received December 18, 2021 | Accepted February 02, 2022

**ABSTRACT**

*The way to minimize traffic problems, especially the safety and comfort of roads is to build medians. The median as part of the road geometry is a physical separator of traffic lanes that serves to relieve traffic conflicts from the opposite direction. So, it improves traffic safety with a reversal facility, analyzes the average travel time of vehicles when making a U-turn, and analyzes the level of service on the Jendral Basuki Rachmat road. Therefore, this research aimed to determine the volume of traffic on Jenderal Basuki Rachmat road, determine the average speed of turning vehicles and determine the service level on Jalan Jenderal Basuki Rachmat Road. This research used the 1997 MKJI methodology. The research results explained that the largest volume of vehicles that make U-turns was 19966 vehicles/hour with a road service level of D. The average travel time of vehicles was 15.3 seconds with a queue length of 50 meters for a U-turn.*

**Keywords:** U-Turn, Speed, Service Level

**1. PRELIMINARY**

Vehicle users expect convenient and safe facilities on the road network. The way to minimize the problem of traffic movement, especially the safety and comfort of roads is to build a median. In median planning, there is a median opening to simplify the vehicle to change the direction, such as a reversal movement or a U-turn movement. One effect of the U-turn motion is the speed of the vehicle slowing down or stopping.

Jendral Basuki Rachmat road is a road that is in front of the Basura Mall, Basura City apartments and the Gembrong market. This

road is in Jatinegara, East Jakarta. Jendral Basuki Rachmat road, which is right in front of Basura Mall and Basura City Apartments, is a 4-lane, 2-way, and 2-turn road on the west before the intersection and on the east before the intersection.

The research objectives were:

1. To determine the effect of the U-turn facility on Jenderal Basuki Rachmat road on the traffic flow characteristics on Jenderal Basuki Rachmat road.
2. To find out the number of vehicles turning around at the U-turn facility on Jenderal Basuki Rachmat road.

- To find out the average travel time of vehicles when making a U-turn on Jenderal Basuki Rachmat road.

## 2. THEORETICAL FRAMEWORK

### Transportation

Transportation is the business and activity of transporting or bringing goods and/or people from one place to another.

### Road

In the Act No. 38 of 2004 article 1 paragraph (4), the definition of road is land transportation infrastructure covering all parts of the road, such as complementary buildings and traffic equipment on the surface of the land and water, and above the water surface, except for railroads, lorries and cable roads.

In the Indonesian Highway Capacity Manual (1997), road types are divided into:

- A two-lane two-way street without a median (2/2 UD).
- A four-lane and two-way street.
  - Undivided / without median (4/2 UD).
  - Divide / by median (4/2 D).
- A six-lane two-way street divided by a median (6/2 D).
- One way street (1-3/1).

Table 1. Median Minimum Width

| Planning Class |         | Standard Minimum Width (m) | Specific Minimum Width (m) |
|----------------|---------|----------------------------|----------------------------|
| TYPE I         | Class 1 | 2.5                        | 2.5                        |
|                | Class 2 | 2.0                        | 2.0                        |
| TYPE II        | Class 1 | 2.0                        | 1.0                        |
|                | Class 2 | 2.0                        | 1.0                        |
|                | Class 3 | 1.5                        | 1.0                        |
|                | Class 3 | 1.5                        | 1.0                        |

Source: "Geometric Planning Standards for Urban Streets" (1992). Directorate General of Highways"

Table 2. Minimum Width of Median Opening (raising/lowering)

| Road Function    | Minimum width (m) |                 |           |
|------------------|-------------------|-----------------|-----------|
|                  | Median            | Inside Shoulder | Road-Edge |
| Arteries         | ≥ 5,00            | 0,50            | 0,25      |
| Collector/ Local | ≥ 4,00            | 0,50            | 0,25      |

Source: Construction and Building Guidelines (2004), Department of Settlement and Regional Infrastructure.

Table 3. Minimum Distance Between Opening and Opening Width

| Road Function | Out of town               |   | Urban                     |         |                                    |
|---------------|---------------------------|---|---------------------------|---------|------------------------------------|
|               | Opening Distance (dl, km) |   | Opening Distance (dl, km) |         | Opening Width (d <sup>2</sup> , m) |
|               |                           |   | Sub-urban                 | In town |                                    |
| Arteries      | 5                         | 7 | 2.5                       | 0.5     | 4                                  |
| Collector     | 3                         | 4 | 1.0                       | 0.3     | 4                                  |

Source: Construction and Building Guidelines (2004) Department of Settlement and Regional Infrastructure.

### Traffic Flow

Traffic flow is the number of motorized vehicles that pass the road and is measured in certain time intervals. Traffic volume is defined as the number of vehicles that passing a point on the highway for a unit of time.

The objectives of determining traffic volume include:

- Determination of traffic flow on a road.
- The tendency of road use.
- Distribution of traffic on a road system.

In general, the traffic volume unit uses the Average Daily Traffic (abbreviated LHR).

Table 4. Emp Value for Divided and One-Way Urban Roads

| Road Type:<br>One-way Street and divided road | Traffic Flow per lane (Vehicles/hour) | Emp |      |
|---|---------------------------------------|-----|------|
|   |                                       | HV  | MC   |
| Two Lane One Way (2/1)                        | 0                                     | 1.3 | 0.40 |
| Four Lane One Way (4/2D)                      | ≥ 1050                                | 1.2 | 0.25 |
| Three Lane One Way (3/1)                      | 0                                     | 1.3 | 0.40 |
| Six-lanes divided (6/2D)                      | ≥ 1100                                | 1.2 | 0.25 |

Source: Indonesian Highway Capacity Manual (1997)

**Vehicle Characteristics**

Table 5. Description Of Passenger Car Unit (abbreviated PCU) Value

| Transportation Type | Passenger Car Unit Value (pcu/hour) |
|---------------------|-------------------------------------|
| Heavy vehicle (HV)  | 1.3                                 |
| Light Vehicle (LV)  | 1.0                                 |
| Motorcycle (MC)     | 0.5                                 |

Source: Indonesian Highway Capacity Manual (1997)

**Capacity**

Table 6. Base Capacity (Co) For Urban Road

| No. | Road Type                     | Basic Capacity (pcu/hour) | Note     |
|-----|-------------------------------|---------------------------|----------|
| 1.  | Four lanes divided or one way | 1650                      | per lane |

| No. | Road Type           | Basic Capacity (pcu/hour) | Note          |
|-----|---------------------|---------------------------|---------------|
| 2.  | Four-lane undivided | 1500                      | per lane      |
| 3.  | Two-lane undivided  | 2900                      | two-way total |

Source: Indonesian Highway Capacity Manual 1997

Table 7. FCw Adjustment for the Effect of Traffic Lane Width on Urban Roads

| Nb. | Road Type                            | Effective traffic lane width (Wc) (M) | FWC  |
|-----|--------------------------------------|---------------------------------------|------|
| 1.  | Four-lanes divided or one-way street | Per Lane                              |      |
|     |                                      | 3,00                                  | 0,92 |
|     |                                      | 3,25                                  | 0,96 |
|     |                                      | 3,50                                  | 1,00 |
|     |                                      | 3,75                                  | 1,04 |
| 2.  | Four-lanes undivided                 | Per Lane                              |      |
|     |                                      | 3,00                                  | 0,91 |
|     |                                      | 3,25                                  | 0,95 |
|     |                                      | 3,50                                  | 1,00 |
|     |                                      | 3,75                                  | 1,05 |
| 3.  | Two-lanes undivided                  | Per Lane                              |      |
|     |                                      | 5                                     | 0,56 |
|     |                                      | 6                                     | 0,87 |
|     |                                      | 7                                     | 1    |
|     |                                      | 8                                     | 1,14 |
|     |                                      | 9                                     | 1,25 |
|     |                                      | 10                                    | 1,29 |
|     |                                      | 11                                    | 1,34 |

Source: Indonesian Highway Capacity Manual 1997

Table 8. FCsp Capacity adjustment factor for directional separator

| Direction Separator SP %-% |                | 50-50 | 60-40 | 70-30 | 80-20 | 90-10 | 100-0 |
|----------------------------|----------------|-------|-------|-------|-------|-------|-------|
| FCsp                       | Two-lanes 2/2  | 1.00  | 0.94  | 0.88  | 0.81  | 0.76  | 0.76  |
|                            | Four-lanes 4/2 | 1.00  | 0.97  | 0.94  | 0.91  | 0.88  | 0.85  |

Source: Indonesian Highway Capacity Manual 1997

Table 9. FCsf Adjustment Factor for the Effect of Side Friction and Curb-Friction Distance on Urban Road and Shoulder Capacity

| Nb. | Road Type                     | Side Friction Class (SFC) | Adjustment factor for side friction and curb-barrier distance (Wk) |       |       |      |
|-----|-------------------------------|---------------------------|--|-------|-------|------|
|     |                               |                           | Curb-friction distance Wk (m)                                      |       |       |      |
|     |                               |                           | <0,5 M   | 1,0 M | 1,5 M | >2M  |
| 1.  | Four-lanes divided (4/2 D)    | Very low                  | 0,95   | 0,97  | 0,99  | 01   |
|     |                               | Low                       | 0,94   | 0,96  | 0,98  | 1,00 |
|     |                               | Moderate                  | 0,91   | 0,93  | 0,95  | 0,98 |
|     |                               | High                      | 0,86   | 0,89  | 0,92  | 0,95 |
|     |                               | Very high                 | 0,81   | 0,85  | 0,88  | 0,92 |
| 2.  | Four-lanes undivided (4/2 UD) | Very low                  | 0,95   | 0,97  | 0,99  | 1,01 |
|     |                               | Low                       | 0,93   | 0,95  | 0,97  | 1,00 |
|     |                               | Moderate                  | 0,90   | 0,92  | 0,95  | 0,97 |
|     |                               | High                      | 0,84   | 0,87  | 0,90  | 0,93 |
|     |                               | Very high                 | 0,77   | 0,81  | 0,85  | 0,90 |

|    |  |           |      |      |      |      |
|----|--|-----------|------|------|------|------|
| 3. | Two-lanes undivided (2/2 UD) or one way street | Very low  | 0,93 | 0,95 | 0,97 | 0,99 |
|    |  | Low       | 0,90 | 0,92 | 0,95 | 0,97 |
|    |  | Moderate  | 0,86 | 0,88 | 0,91 | 0,94 |
|    |  | High      | 0,78 | 0,81 | 0,84 | 0,88 |
|    |  | Very high | 0,68 | 0,72 | 0,77 | 0,82 |

(Source: Indonesian Highway Capacity Manual 1997)

Table 10. FCcs Adjustment Factor for the Effect of City Size on Urban Road Capacity

| Nb. | City Size (Million Population) | Adjustment Factor for city size FCcs |
|-----|--------------------------------|--------------------------------------|
| 1.  | <0.1                           | 0.86                                 |
| 2.  | 0.1-0.5                        | 0.90                                 |
| 3.  | 0.5-1.0                        | 0.94                                 |
| 4.  | 1.0-3.0                        | 1.00                                 |
| 5.  | >3.0                           | 1.04                                 |

(Source: Indonesian Highway Capacity Manual 1997)

### Speed

According to Hobbs (1995), speed is the main parameter to describe traffic flow and travel speed recognized in kilometers per hour (km/hour).

### Degree of Saturation



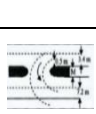
The degree of saturation (DS) is the ratio of road flow to capacity as the main factor in determining the performance level of intersections and road segments. The DS value indicates whether the road segment has a capacity problem or not.

### U-Turn

In general, the U-turn condition is used for the rotation of the vehicle. However, there are U-turns that are prohibited from being used, for example there are traffic signs with assistive tools such as chain iron stakes. It can be found on the freeway. The U-turn

function is only for officers or during an emergency. According to the type of movement, U-Turn is divided into 3 types, namely single U-turn, double U-turn, and multiple U-turns.

Table 11. Minimum Width of Median Opening Design For U-Turn

| Movement type                 |   | Minimum Median Opening Width (m) For Design Vehicles |       |    |     |       |
|-------------------------------|---|--|-------|----|-----|-------|
|                               |   | P  | WB-40 | SU | BUS | WB-50 |
|                               |   | Design Vehicle Length                                |       |    |     |       |
|                               |   | 5.7  | 15    | 9  | 12  | 16.5  |
| Inside lane into outside lane |    | 9  | 18    | 19 | 19  | 21    |
| Inside lane into outside lane |   | 6  | 15    | 15 | 16  | 18    |
| Inside lane into outside lane |  | 2  | 12    | 12 | 12  | 15    |

**Side Friction**

According to the 1997 Indonesian Highway Capacity Manual (abbreviated MKJI), side friction is the impact of side activities on the road segment on traffic roads, such as:

1. Pedestrians who walk or cross along a road segment.
2. Public transport and other vehicles that stop and park.
3. Motorized vehicles in and out of/to the land side/side of the road.
4. Slow moving traffic.

Table 12. FCsf Adjustment Factor for the Effect of Side Friction and Shoulder Width on Urban Road and Shoulder Capacity

| Nb. | Road Type                                      | Side Friction Class (SFC) | Adjustment factor for side friction and shoulder width (FCsf) |      |       |      |
|-----|--|---------------------------|---|------|-------|------|
|     |  |                           | Average effective shoulder width Ws (M)                       |      |       |      |
|     |  |                           | <0,5M   | 1,0M | 1,5 M | >2M  |
| 1.  | Four-lanes divided (4/2 D)                     | Very low                  | 0.96  | 0.98 | 1.01  | 1.03 |
|     |  | Low                       | 0.94  | 0.97 | 1.00  | 1.02 |
|     |  | Moderate                  | 0.92  | 0.95 | 0.98  | 1.00 |
|     |  | High                      | 0.88  | 0.92 | 0.95  | 0.98 |
|     |  | Very high                 | 0.84  | 0.88 | 0.92  | 0.96 |
| 2.  | Four-lanes undivided (4/2 UD)                  | Very low                  | 0.96  | 0.99 | 1.01  | 1.03 |
|     |  | Low                       | 0.91  | 0.97 | 1.00  | 1.02 |
|     |  | Moderate                  | 0.92  | 0.95 | 0.98  | 1.00 |
|     |  | High                      | 0.87  | 0.91 | 0.94  | 0.98 |
|     |  | Very high                 | 0.80  | 0.86 | 0.90  | 0.95 |
| 3.  | Two-lanes undivided (2/2 UD) or one way street | Very low                  | 0.94  | 0.96 | 0.99  | 1.01 |
|     |  | Low                       | 0.92  | 0.94 | 0.97  | 1.00 |
|     |  | Moderate                  | 0.89  | 0.92 | 0.95  | 0.98 |
|     |  | High                      | 0.82  | 0.86 | 0.90  | 0.95 |
|     |  | Very high                 | 0.73  | 0.79 | 0.85  | 0.91 |

Table 13. Side Friction Class and Shoulder Width on Urban Road Capacity with Shoulders

| Occurrence weighted frequency | Symbol  | Side Friction Class |    |
|-------------------------------|---|---------------------|----|
|                               |   | C                   | D  |
| A                             | B   | C                   | D  |
| <50                           | Plantations/areas less developed, no activities | Very low            | VL |
| 50-100                        | Few settlements and low activities              | Low                 | L  |
| 150-200                       | Countryside, settlement activities              | Mode rate           | M  |
| 250-249                       | Countryside, some market activities             | High                | H  |
| >350                          | Near cities, market/commercial activities       | Very high           | VH |

(Source: Indonesian Highway Capacity Manual 1997)

Table 14. Occurrence weight for side friction

| Pedestrian | Public transportati on or other vehicles stop | Vehicle entering or exiting the road side | Slow vehicle |
|------------|---|---|--------------|
| 0.5        | 1.0   | 0.7                                       | 0.7          |

(Source: Indonesian Highway Capacity Manual 1997)

### Road Service Level

The road service level is performance of roads or intersections size that is calculated based on the level of road users, speed, density, and obstacles that occur.

Table 15. Road service level characteristics

| Service Level | Description   | Degree of Saturation (DS) |
|---------------|---|---------------------------|
| A             | Free flow conditions; low volume and speed                    | 0.00-0.20                 |
| B             | In the stable current zone. Speed slightly limited by traffic | 0.20-0.44                 |
| C             | In stable flow zone but speed controlled by traffic           | 0.45-0.74                 |
| D             | Current Approaching unstable. Operation speed                 | 0.75-0.854                |
| E             | Different - sometimes stops, volume                           | 0.85-1.00                 |
| F             | Low, volume below capacity, queue                             | >1.00                     |

### RESEARCH METHODS

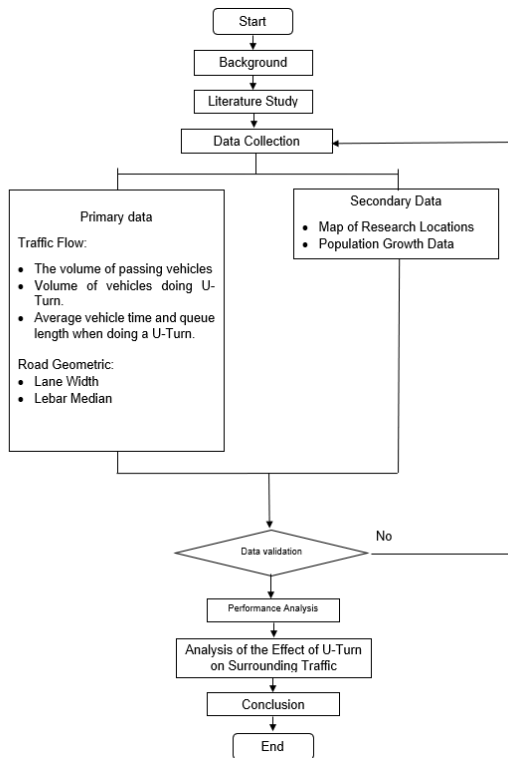


Figure 1. Research methodology stages

Table 16. Traffic Volume Data for Weekday I (vehicle/hour)

| Time             | Number of Vehicles (vehicles/hour) Jalan Jenderal Basuki Rachmat Street |      |    |           |      |    |
|------------------|---|------|----|-----------|------|----|
|                  | East-West   |      |    | West-East |      |    |
|                  | MC  | LV   | HV | MC        | LV   | HV |
| <b>Weekday I</b> |   |      |    |           |      |    |
| <b>Morning</b>   |   |      |    |           |      |    |
| 07.00-08.00      | 4472  | 2922 | 97 | 2583      | 978  | 77 |
| 08.00-09.00      | 2896  | 1106 | 62 | 2821      | 1304 | 77 |
| <b>Afternoon</b> |   |      |    |           |      |    |
| 16.00-17.00      | 3444  | 926  | 79 | 3803      | 1009 | 92 |
| 17.00-18.00      | 3424  | 1072 | 58 | 3828      | 1273 | 97 |

Source: Field survey

### 3. RESULTS AND DISCUSSION

#### Traffic Volume

The total observation time was 4 hours per day for three days per point. Observations were conducted at 07.00 - 09.00 WIB, and 16.00 - 18.00 WIB.

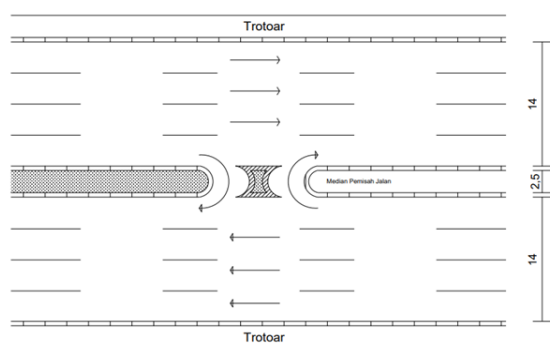


Figure 1. Geometric U-Turn Jl. General Basuki Rachmat

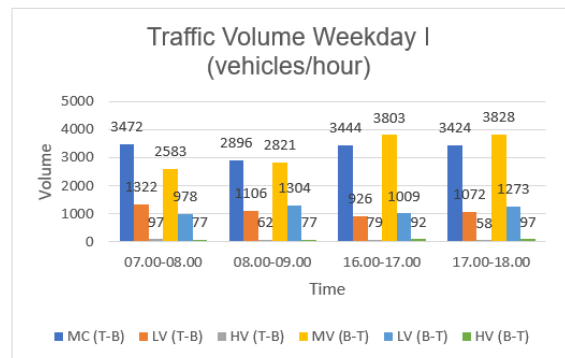


Figure 2. Traffic Volume Weekday I (vehicles/hour)

Table 17. Traffic Volume Data for Weekday II (vehicles/hour)

| Time        | Number of Vehicles (vehicles/hour) Jalan Jenderal Basuki Rachmat Street |      |    |           |      |     |
|-------------|---|------|----|-----------|------|-----|
|             | East-West   |      |    | West-East |      |     |
|             | MC  | LV   | HV | MC        | LV   | HV  |
| Weekday II  |   |      |    |           |      |     |
| Morning     |   |      |    |           |      |     |
| 07.00-08.00 | 2479  | 864  | 67 | 2398      | 1394 | 98  |
| 08.00-09.00 | 2882  | 1087 | 52 | 2562      | 878  | 93  |
| Afternoon   |   |      |    |           |      |     |
| 16.00-17.00 | 3292  | 800  | 89 | 3253      | 1334 | 106 |
| 17.00-18.00 | 3401  | 1023 | 92 | 3657      | 807  | 86  |

Source: Field survey

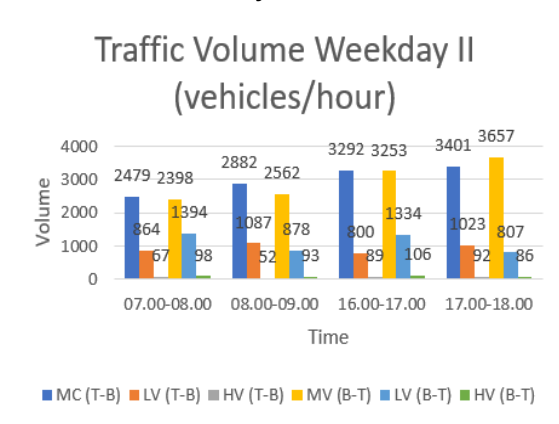


Figure 3. Traffic Volume Weekday II (vehicles/hour)

Table 18. Traffic Volume Data for Weekday III (vehicles/hour)

| Time        | Number of Vehicles (vehicles/hour) Jalan Jenderal Basuki Rachmat Street |      |    |           |      |     |
|-------------|---|------|----|-----------|------|-----|
|             | East-West   |      |    | West-East |      |     |
|             | MC  | LV   | HV | MC        | LV   | HV  |
| Weekday III |   |      |    |           |      |     |
| Morning     |   |      |    |           |      |     |
| 07.00-08.00 | 2147  | 792  | 75 | 2173      | 1356 | 107 |
| 08.00-09.00 | 2474  | 1022 | 84 | 2367      | 923  | 86  |
| Afternoon   |   |      |    |           |      |     |
| 16.00-17.00 | 3135  | 958  | 77 | 3325      | 877  | 82  |
| 17.00-18.00 | 3337  | 1104 | 84 | 4918      | 2898 | 119 |

Source: Field survey

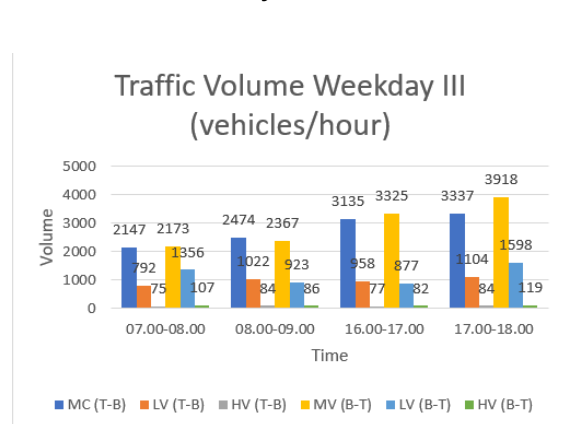


Figure 4. Traffic Volume Weekday III (vehicles/hour)



Table 19. Traffic Volume Data for Weekend (vehicles/hour)

| Time             | Number of Vehicles (vehicles/hour) Jalan Jenderal Basuki Rachmat Street |      |    |           |      |    |
|------------------|---|------|----|-----------|------|----|
|                  | East-West   |      |    | West-East |      |    |
|                  | MC  | LV   | HV | MC        | LV   | HV |
| <b>Weekend</b>   |   |      |    |           |      |    |
| <b>Morning</b>   |   |      |    |           |      |    |
| 07.00-08.00      | 2252  | 1267 | 71 | 2467      | 1543 | 62 |
| 08.00-09.00      | 2397  | 1284 | 76 | 2444      | 1359 | 70 |
| <b>Afternoon</b> |   |      |    |           |      |    |
| 16.00-17.00      | 3357  | 1238 | 61 | 3182      | 1211 | 68 |
| 17.00-18.00      | 3389  | 1312 | 58 | 3202      | 1351 | 62 |

Source: Field survey

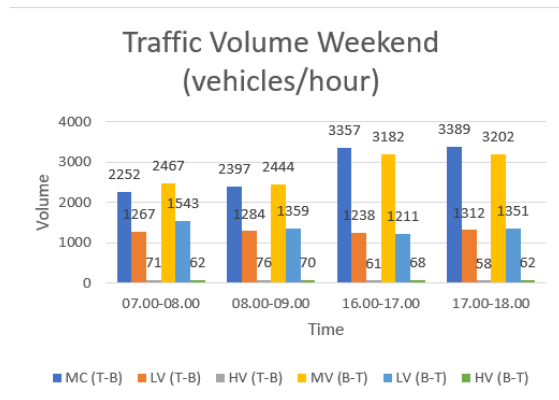


Figure 5. Traffic Volume Weekend (vehicles/hour)

**Calculation of Vehicle Volume from vehicle/hour to smp/hour**

1. Jenderal Basuki Rachmat Street

a. (West-East) Weekday I 08.00-09.00.

$$MC = (4472 \times 0,5) = 1736 \text{ smp/hour}$$

$$LV = (2922 \times 1,0) = 1322 \text{ smp/hour}$$

$$HV = (97 \times 1,3) = 126,1 \text{ smp/hour}$$

$$+ 5284,1 \text{ smp/hour}$$

b. (West-East) Weekday III 17.00-18.00.

$$MC = (4918 \times 0,5) = 1959 \text{ smp/hour}$$

$$LV = (2898 \times 1,0) = 1598 \text{ smp/hour}$$

$$HV = (119 \times 1,3) = 154,7 \text{ smp/hour} + 5510,2 \text{ smp/hour}$$

**Traffic Volume Analysis**

The highest traffic volume in the East-West direction on Jalan Jenderal Basuki Rachmat was 5284.1 pcu/hour and the West-East direction was 5510.2 pcu/hour. The peak hour volume in the East-West direction was the weekday I at 08.00-09.00, while the West-East direction was the weekday III at 17.00 - 18.00.

**Road Capacity Data**

The research location was a road segment which consisted of 4 lanes and 2 lanes.

Table 20. Geometric Data of Research Location

| Research Location          | Road Type | Road Width (m) | Median Width (m) | Side friction |
|----------------------------|-----------|----------------|------------------|---------------|
| Jl. Jendral Basuki Rachmat | 4/2 D     | 14             | 2,5              | High          |

Source: Field survey

**Road Capacity Analysis**

Table 21. Road Capacity Calculation

| Research Location          | Adjustment Factor |      |      |      |      |
|----------------------------|-------------------|------|------|------|------|
|                            | Co                | Fcw  | FCsp | FCsf | FCcs |
| Jl. Jendral Basuki Rachmat | 1650              | 1,00 | 1,00 | 0,97 | 1,04 |

Source: Indonesian Highway Capacity Manual

### Road Capacity Calculation

#### 1. Jalan Jenderal Basuki Rachmat

Road sections 4/2 D obtained capacity per lane

$$C = C_o \times FC_w \times FC_{sp} \times FC_{sf} \times FC_{cs}$$

$$= 1650 \times 1.00 \times 1.00 \times 0.96 \times 1.04$$

$$= 1647,36 \text{ smp/hour.}$$

With 4 lanes, the capacity was:

$$C = 4 \times 1664,52 = 6589,44 \text{ smp/hour.}$$

### Degree of Saturation

#### 1. Jenderal Basuki Rachmat Street

##### 1. East-West

$$DS = \frac{Q_{smp}}{C} = \frac{5284,1}{6598,44} = 0,80$$

##### 2. West-East

$$DS = \frac{Q_{smp}}{C} = \frac{5510,2}{6598,44} = 0,83$$

### Analysis of Vehicles Doing U-Turns

Table 22. Number of Vehicles Doing U-Turns from East to West

| Time        | Number of Vehicles (vehicles/hour) |     |    | Total vehicles that do U-turns |
|-------------|------------------------------------|-----|----|--------------------------------|
|             | MC                                 | LV  | HV |                                |
| Weekday I   |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 1167                               | 755 | 27 | 1949                           |
| 08.00-09.00 | 638                                | 365 | 44 | 1047                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 518                                | 297 | 38 | 853                            |
| 17.00-18.00 | 623                                | 375 | 47 | 1045                           |
| Weekday II  |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 428                                | 261 | 43 | 732                            |
| 08.00-09.00 | 709                                | 359 | 29 | 1097                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 496                                | 256 | 38 | 790                            |
| 17.00-18.00 | 637                                | 393 | 27 | 1057                           |
| Weekday III |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 399                                | 307 | 29 | 735                            |
| 08.00-09.00 | 707                                | 338 | 38 | 1083                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 583                                | 307 | 25 | 915                            |
| 17.00-18.00 | 599                                | 387 | 37 | 1023                           |
| Weekend     |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 378                                | 352 | 21 | 751                            |
| 08.00-09.00 | 688                                | 251 | 32 | 971                            |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 594                                | 301 | 28 | 923                            |
| 17.00-18.00 | 596                                | 392 | 33 | 1021                           |

Source: Field survey

Vehicles that did the most U-turns from the East - West were on the weekday I at 08.00-09.00, namely 1949 vehicles.

**Table 23. Number of Vehicles Doing U-Turns from West to East**

| Time        | Number of Vehicles (vehicles/hour) |     |    | Total vehicles that do U-turns |
|-------------|------------------------------------|-----|----|--------------------------------|
|             | MC                                 | LV  | HV |                                |
| Weekday I   |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 681                                | 304 | 28 | 1013                           |
| 08.00-09.00 | 898                                | 431 | 40 | 1369                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 705                                | 323 | 43 | 1071                           |
| 17.00-18.00 | 885                                | 446 | 28 | 1359                           |
| Weekday II  |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 744                                | 433 | 34 | 1211                           |
| 08.00-09.00 | 747                                | 290 | 31 | 1068                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 641                                | 427 | 17 | 1085                           |
| 17.00-18.00 | 895                                | 178 | 37 | 1110                           |
| Weekday III |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 730                                | 421 | 27 | 1178                           |
| 08.00-09.00 | 716                                | 305 | 36 | 1057                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 789                                | 288 | 27 | 1104                           |
| 17.00-18.00 | 1039                               | 910 | 47 | 1996                           |
| Weekend     |                                    |     |    |                                |
| Morning     |                                    |     |    |                                |
| 07.00-08.00 | 582                                | 559 | 19 | 1160                           |
| 08.00-09.00 | 738                                | 632 | 25 | 1395                           |
| Afternoon   |                                    |     |    |                                |
| 16.00-17.00 | 590                                | 463 | 24 | 1077                           |
| 17.00-18.00 | 668                                | 397 | 23 | 1088                           |

Source: Field survey

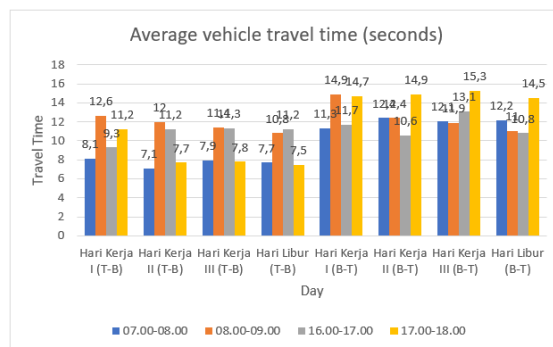
Vehicles that did the most U-turns from the West - East were the weekday III at 17.00-18.00, namely 1996 vehicles.

**Analysis of the Average Travel Time of Vehicles When Doing a U-turn**

**Table 24. Average Travel Time of Vehicles Doing U-Turns**

| Time        | Average vehicle travel time (seconds) |             |             |             |
|-------------|---------------------------------------|-------------|-------------|-------------|
|             | East-West                             |             |             |             |
|             | 07.00-08.00                           | 08.00-09.00 | 16.00-17.00 | 17.00-18.00 |
| Weekday I   | 8,1                                   | 12,6        | 9,3         | 11,2        |
| Weekday II  | 7,1                                   | 12          | 11,2        | 7,7         |
| Weekday III | 7,9                                   | 11,4        | 11,3        | 7,8         |
| Weekend     | 7,7                                   | 10,8        | 11,2        | 7,5         |
| Time        | West-East                             |             |             |             |
|             | 07.00-08.00                           | 08.00-09.00 | 16.00-17.00 | 17.00-18.00 |
| Weekday I   | 11,3                                  | 14,9        | 11,7        | 14,7        |
| Weekday II  | 12,4                                  | 12,4        | 10,6        | 14,9        |
| Weekday III | 12,1                                  | 11,9        | 13,1        | 15,3        |
| Weekend     | 12,2                                  | 11          | 10,8        | 14,5        |

Source: Field survey



**Figure 6. Average vehicle travel time (seconds) from East-West and West-East directions**

The highest average travel time of vehicle turning on Jenderal Basuki Rachmat road from the East - West direction was 12.6 seconds on the weekday I at 08.00-09.00, while from the West - East direction was 15.3 on the weekday III at 17.00- 18.00.

**Vehicle Speed When U-Turn**

1. Jenderal Basuki Rachmat Street (East-West)

Length = 45 meter = 0.045 km

Time = 12,6 second = 0,0035 hour

Speed =  $\frac{0,045}{0,0035} = 12,8$  Km/hour

2. Jenderal Basuki Rachmat Street (West-East)

Length = 45 meter = 0.045 km

Time = 15,3 second = 0,00425 hour

Speed =  $\frac{0,045}{0,00425} = 10,5$  Km/hour

**Vehicle Speed Analysis**

The observations results revealed that the largest number of U-turn vehicles with 1-hour intervals was from the East - West direction on the weekday I at 08.00-09.00 namely 1949 vehicles, while from the West - East direction on the weekday III at 17.00-18.00, namely 1996 vehicles. The number of vehicles that did U-turns and the average maneuvering speed of vehicles that were less than 15 km/hour caused queues and affected the speed of vehicles on the Jenderal Basuki Rachmat road.

### Vehicle Queue Length When U-Turn

Table 25. Vehicle Queue Length When U-Turn

| Time        | Queue Length When Doing U-Turn (Meter) |               |               |               |
|-------------|--|---------------|---------------|---------------|
|             | East-West                              |               |               |               |
|             | 07.00 - 08.00                          | 08.00 - 09.00 | 16.00 - 17.00 | 17.00 - 18.00 |
| Weekday I   | 25                                     | 11            | 17            | 14            |
| Weekday II  | 16                                     | 13            | 10            | 15            |
| Weekday III | 14                                     | 10            | 11            | 12            |
| Weekend     | 6                                      | 9             | 10            | 10            |
| West-East   |  |               |               |               |
| Weekday I   | 8                                      | 12            | 14            | 20            |
| Weekday II  | 7                                      | 13            | 16            | 19            |
| Weekday III | 6                                      | 10            | 18            | 50            |
| Weekend     | 8                                      | 10            | 15            | 17            |

Source: Field survey

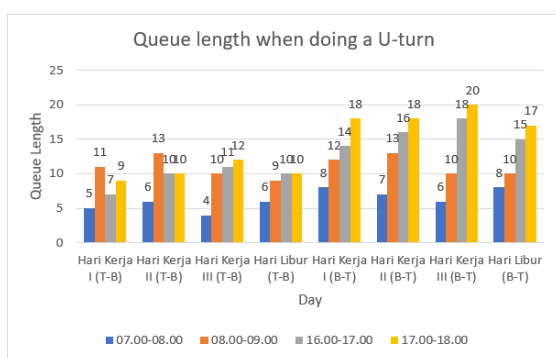


Figure 7. Queue length when doing U-turns from East-West and West-East directions

The highest queue length for vehicles in the East-West direction was the weekday I of 25 meters, while in the West-East direction was the weekday III of 50 meters which was classified as severe queue conditions.

### Road Service Level Using Ratio V/C

Table 26. Road Service Level

| Nb. | Location                      | Volume (V) (smp/hour) | Capacity (C) (smp/hour) | V/C  | Service Level |
|-----|-------------------------------|-----------------------|-------------------------|------|---------------|
| 1   | Jalan Jenderal Basuki Rachmat | 5510,2                | 6589,44                 | 0,83 | D             |

### U-turn analysis

The observations results showed that the data on the number of vehicles doing U-turns on Jenderal Basuki Rachmat street. The highest number of vehicles doing U-turns could be seen in tables 4.7 and 4.8. At intervals of 1 hour, in the East - West

direction the highest number was the weekday I at 08.00-09.00, namely 1949 vehicles, while in the West - East direction was the weekday III at 17.00-18.00, namely 1996 vehicles. The number of vehicles that did U-turns and the average maneuvering speed of vehicles that were less than 15 km/hour caused queues and affected the speed of vehicles on the Jenderal Basuki Rachmat street. The highest vehicle queue length could be seen in table 4.10, which was 50 meters in the West-East direction at 17.00-18.00. Based on the analysis of the effect of the U-turn on the Jenderal Basuki Rachmat street characteristics, the authorities should improve traffic management.

### 4. CONCLUSION

Based on the data analysis results, several conclusions are obtained, including:

1. The U-turn facility on Jenderal Basuki Rachmat street affects the traffic flow characteristics. Vehicles that do U-turns at 07.00 and 18.00 as rush hour and high side friction create queues and affect vehicle speed on Jenderal Basuki Rachmat street. The characteristics of traffic flow on Jenderal Basuki Rachmat street are level D with a v/c value of 0.83.
2. The highest number of vehicles using the U-turn facility on Jenderal Basuki Rachmat street from the East-West direction is 1949 vehicles per hour, while from the West-East direction is 1996 vehicles per hour.
3. The highest average vehicle travel time when doing a U-turn on Jalan Jenderal Basuki Rachmat from the East-West direction is 12.6 seconds, while from the West-East direction it is 15.3 seconds.
4. The highest vehicle queue length when doing a U-turn in the East-West direction is 25 meters, while the West-East direction is 50 meters.

## REFERENCES

- [1] Adris P. A. & Sarwono S. A. (2008). Pengaruh Pergerakan U-Turn (Putar Balik Arah) Terhadap Kecepatan Arus Lalu Lintas Menerus.
- [2] Caroline, Jenny & Winaya, Amrita. (2019). Analisis Putaran Balik (U-Turn) Terhadap Tingkat Pelayanan Ruas Jalan Raya Waru Sidoarjo.
- [3] Direktorat Jenderal Bina Marga. (1997). Manual Kapasitas Jalan Indonesia. Jakarta.
- [4] Hobbs. (1995). Perencanaan Dan Teknik Lalu Lintas. Yogyakarta.
- [5] Kasan, Muhammad., Mashuri, & Listiawati, Hilda. (2005). Pengaruh U-Turn Terhadap Karakteristik Arus Lalu Lintas Di Ruas Jalan Kota Palu (Studi Kasus Jl. Moh. Yamin Palu).
- [6] Kamaluddin, Rustian (2003). Ekonomi Transportasi Karakteristik Teori, Dan Kebijakan. Jakarta.
- [7] Koloway, B. S. (2009). Kinerja Ruas Jalan Perkotaan Jalan Prof. Dr. Satrio, Dki Jakarta. Journal Of Regional And City Planning.
- [8] Mardinata, L. A. (2014). Tugas Akhir Pengaruh U-Turn (Putar Balik Arah) Terhadap Kinerja Arus Lalu Lintas Ruas Jalan Raden Eddy Martadinata.
- [9] Prasetyo, Harwidyo Eko & Santoso, Tri. (2020). Analisis Kinerja U-Turn (Studi Kasus U-Turn Di Itc Jalan Letjen Soepono, Jakarta). Jakarta.
- [10] Purba, E. A. & Harianto, J. (2014). Pengaruh Gerakan U-Turn Pada Bukaannya Median Terhadap Karakteristik Arus Lalu Lintas Di Ruas Jalan Kota.
- [11] Putar, (2013). Kajian Balik U-Turn Terhadap Kemacetan Ruas Jalan Di Pagar Alam, Dan Kota Bandar (Studi Kasus Ruas Jalan Teuku Umar).
- [12] Rangkuti, Nuril Mahda. (2012). Analisa Pengaruh Putaran Balik (U-Turn) Terhadap Kinerja Ruas Jalan (Studi Kasus ).
- [13] Sukirman, Silvia. (1999). Dasar-Dasar Perencanaan Geometrik Transportasi. Bandung.
- [14] Utami, Yuwita Tri., Ariyadi, Teddy & Mayuni, Siti. (2017). Kajian Putar Balik (U-Turn) Terhadap Arus Lalu Lintas (Studi Kasus : Jalan Gajah Mada Pontianak).

